SLR International Corporation
99 Realty Drive, Cheshire, Connecticut, 06410

ADDENDUM NO. 2

SMITHS FALLS HYDROELECTRIC PROJECT

Stowe Electric Department
Stowe, Vermont

SLR #146.V17412.00001
Original Bid Package Issued December 15, 2025
Addendum No. 2 Issued January 23, 2026

This Addendum No. 2 includes clarifications and modifications to the above-referenced bid package. Changes reflected herein shall supersede all previous bid package documentation.

A. PRE-BID MEETING – RESPONSE TO QUESTIONS

A mandatory Pre-Bid Meeting was held at the project site on January 6, 2026. The following questions were noted during the meeting. Responses and clarifications were not available for all questions prior to issuing Addendum #1, however additional information and responses to those questions have been provided below. Changes reflected herein shall supersede all previous bid package documentation.

Q1. Are elevations of the sediment upstream of the dam available?

R1. Spot elevations for the top of sediment upstream of the dam have been provided, see Sheet EX-02 revised to January 23, 2026 provided as Attachment A.

Q2. Are weights of the CHC components available?

R2. Weights and bill of materials are provided on the preliminary transportation drawing, see Attachment B.

Q3. What testing of the hydroelectric components will be required?

R3. Testing requirements for the hydroelectric components are provided in CHC's Commissioning Overview Plan – Draft r1, see Attachment C.

Q4. Are weights of the Obermeyer components available?

R4. Weights and bill of materials are provided in the following table:

Part Name	Quantity	Weight (lbs)	Total Weight (lbs)
Gate Panel	8	1709	13678
Hinge Retainer 3 Hole	88	3	229
Hinge Retainer 4 Hole	56	4	196
Inter Panel Seal Retainer	28	6	176
Restraining Strap Clamp	48	20	941
Abutment Seal Retainer	4	6	25

Part Name	Quantity	Weight (lbs)	Total Weight (lbs)
Main Anchor Assembly	8	317	2537
Restraining Strap Embed Assembly	8	19	153
Main Clamp Bar End	16	36	576
Main Clamp Bar Middle	32	39	1248
Abutment Plates L&R	2	349	697
Bladder	8	5000	40000
Restraining Strap	24	10	240
Inter Panel Seal	7	15	105
Abutment Seal L&R	2	15	30

Q5. Are the specifications for the intake and low-level slide gates available? Will alternative slide gate manufactures be accepted?

R5. Specifications for American-made slide gates have been provided in Attachment D. Alternative products that meet the specifications will be considered upon review and approval by the Project Engineer.

Q6. Will all new concrete require coloring or staining? Or just the west foundation wall of the historic mill?

R6. The purpose of tinting new concrete to meet the historic preservation permit requirements is to soften the visual impact to the Seaver Sawmill from new concrete and/or repaired concrete that will be located adjacent to the mill. The Smith's Falls Dam does not require a tint or wash to create this visual distinction.

The contractor shall tint or wash any new concrete poured by the contractor or its subcontractors that will be exposed and/or will sit above the water line. The Sika French Grey (C14) product as shown on the SikaColor-120 G Granular Integral Colors color chart shall be used. A revised Section 03 36 00 – Integral Colored Concrete of the Technical Specifications will be provided to the selected contractor.

Any new concrete poured as part of this project that sits below the water line and/or is not exposed will not be tinted. This includes any footings, the bottom slab of the restored sluiceway intake, sluiceway extension, and within the turbine chamber of the new powerhouse. In addition, the rehabilitated spillway of the dam and the south abutment will not require coloring or tinting.

Any tint or wash applied to concrete poured for the West Wall repair shall only apply to new concrete that is visible, and shall not be required for the footings.

Any repair and repointing required to repair the foundation of the west end of the Seaver Sawmill shall follow applicable NPS guidance available at:

https://www.nps.gov/orgs/1739/preservation-briefs.htm,

including Repointing Mortar Joints in Historic Masonry Buildings and The Maintenance, Repair, and Replacement of Historic Cast Stone. Additional photos of the interior of the mills foundation have been provided electronically.

Q7. Will a unit bid price (per square foot) for stone masonry repairs be added to the bid form rather than including in the lump sum bid for the mill foundation repairs?

R7. Yes, an updated Bid Form has been provided as Attachment E. A Unit Price shall be provided for Stone Masonry Repair / Repointing, and shall be paid based on the unit price. The cost associated with Stone Masonry Repair / Repointing shall be omitted from the lump sum cost for Bid Item 20 – Mill Repairs – West Wall & Foundation. In addition, a Unit Price shall be provided for Cast-In-Place Concrete with coloring, which shall include all materials and work for the construction of integral colored cast-in-place concrete structures as accepted per cubic yard as measured in place.

Q8. What testing is required for the cast-in-place concrete? For the rock anchors?

R8. Refer to the responses provided in Addendum 1 for more information related to testing requirements for cast-in-place concrete. Refer to the response to Question 36 through Question 39 in Section B below for more information related to rock anchor testing requirements.

B. RESPONSE TO BIDDER QUESTIONS

The following is a list of questions and requests for information that were submitted in writing by interested bidders that attended the Pre-Bid Meeting.

Q1. Please confirm that the owner has pre-purchased the hydroelectric turbine and generator from Canadian Hydro Components. Additionally, please provide the following supplemental information:

a. The bill of materials for all components (specifically the draft tube fabrications, the generator frame, the runner and the generator).

b. Delivery dates for all components.

c. Estimated weights for all components.

R1. Refer to the response to Question 2 in Section A above for weights and bill of materials.

The delivery date shall be coordinated between the contractor and Canadian Hydro Components to ensure that components arrive at the site when needed. A lead time of approximately 5 days is required for shipment and delivery. The Contractor provided work schedule will be shared with the manufacturer upon award of the bid.

Q2. Please confirm that the owner has pre-purchased the spillway crest gates from Obermeyer Hydro, Inc. Additionally, please provide the following supplemental information:

a. The bill of materials for all components.

b. Delivery dates for all components.

c. Estimated weights for all components.

R2. Refer to the response to Question 4 in Section A above for weights and bill of materials.

The delivery date shall be coordinated between the contractor and Obermeyer to ensure that components arrive at the site when needed. A lead time of approximately 14 days is required for shipment and delivery. The Contractor provided work schedule will be shared with the manufacturer upon award of the bid.

Q3. Please provide a specification section for the Fontaine Aquanox slide gates as shown at the intake and the low-level outlet at the dam.

R3. Refer to the response to Question 5 in Section A for more information.

Q4. The drawings, specifically PR-02 indicate various areas where uncontaminated sediment needs to be removed, upstream of the dam, the existing mill intake and the existing sluiceway. We are unable to determine the number of materials that need to be removed, however. Would the owner provide an estimated quantity or provide an allowance so that all bidders are carrying the same assumption?

R4. Refer to the response to Question 1 in Section A for more information.

Q5. Please note the trashrack details as shown on STR-04 and STR-05, and provide the following:

a. Provide connection details for the SS frame to the intake base slab, intake walls and intake deck.

b. Confirm the size of trash rack bars (1/4"x 1/4"), the material and the connection details to the frame.

R5. a. The trash rack frame shall be connected to the concrete using ¾" Hilti HY 150 expansion bolts with minimum 3-inch embedment spaced a maximum of 12-inches O.C.

b. The bars of the trash rack shall be ¼" thick by 1" deep.

Q6. Who is responsible for providing a baseline layout for the new structures, intake, sluiceway, powerhouse, tailrace and the dam?

R6. The contractor is responsible for baseline layout with review by the project engineer. CAD drawings will be provided to the contractor awarded the bid.

Q7. Please provide framing, grating and connection details for the 7'-6" x 8'-8" opening in the new sluiceway, as shown on STR-01.

R7. Grating shall be connected to steel angle with clips per grating manufacturer's recommendations. Grating shall be Standard Duty Welded Rectangular Bar, Serrated Surface, No. GW-150 (19-W-4), and shall be galvanized steel, by McNichols or approved equal. Shop drawings shall be required.

Q8. Please provide framing, grating and connection details for the grated opening in the new sluiceway, as shown on STR-04.

R8. Additional information is provided in SK-1, see Attachment F.

Q9. The W8x35 beams that support the powerhouse slab, are shown to sit in a beam seat in the foundation wall. Is there any additional support, anchoring or other that is required for these beams? Additionally, what is the material and/or coating for these beams?

R9. Use ¾" anchor bolt with 6" minimum embedment, one per end (2 per beam). Anchors shall be placed 3 inches from the inside face of the wall.

Q10. Please note the proposed intake details as shown on STR-04 and advise on the following:

a. Please confirm that the intent is the re-use the existing sluiceway/intake foundation walls (section 4/STR-04), which will be extended as a retaining wall on the north side of the intake (section 1/STR-01) and a single faced wall on the south side.

b. Please provide details and dimensions for the single faced wall on the south side of the intake.

c. Section 3 and 4, indicate the construction of a new base slab for the intake, however the dimensions are not provided. Please provide the dimensions or the delineation between new and existing for the new slab.

d. The plan view of the intake shows dashed lines which appear to indicate some sort of trailing wall below the proposed top slab, downstream of the new column. Please provide dimensions and reinforcement details for this wall.

e. The concrete column is shown in plan on STR-04 and in elevation on STR-05, specifically in the elevation named 'Intake with Trash Rack'. In this elevation, it appears that the column is centered on the 22'-8" trash rack, however in plan view the placement is offset substantially from the racks. Please advise.

f. Provide connection details for the SS frame to the intake base slab, intake walls, intake deck and the column.

g. Confirm the size of trash rack bars (1/4"x 1/4"?), the material and the connection details to the frame?

R10. a. Existing walls will be re-used. A new top and bottom slab will be installed at the proposed intake will be installed as shown on the plans and details. The proposed bottom slab shall have waterstops installed at the face of the existing walls.

b. The wall should be constructed to join the face of existing concrete mill structure with the inside face of sluiceway wall. Thickness of the wall may vary based on field conditions. Plans are drawn to scale.

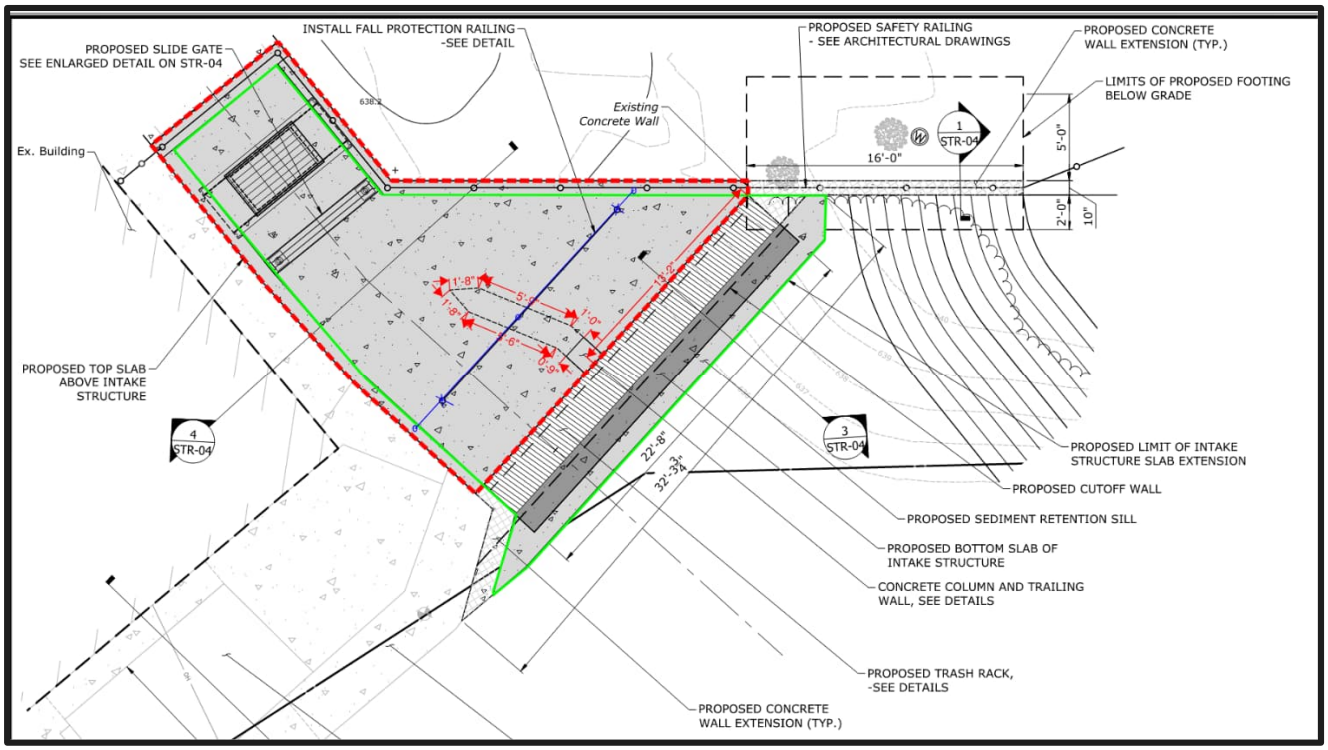
c. See the sketch provided below for the plan area of base slab outlined in green. The plan area outlined in dashed red represents extent of top slab.

d. The trailing wall shall have vertical reinforcing on each face, #5 @ 12" o.c. and horizontal reinforcing on each face, #5 @ 12" o.c. extending 12" into concrete column. Wall is 18" thick. See sketch for length dimension.

e. See the sketch provided below for the horizontal placement of pier.

f. See the response to Question #5 in Section B for more information.

g. See the response to Question #5 in Section B for more information. All pieces shall be stainless steel, and bars shall be welded to the frame.



Q11. Safety railings are shown in various plan drawings, on the east and west sides of the mill, placed on the new sluiceway top slab, and note to refer to the Architectural drawings for details. Please provide the following:

a. Post spacing and connection details to the structure.

b. Details for the 'lockable gate' as shown in plan on SP-01.

c. Please confirm that the finish is 'galvanized' and not painted.

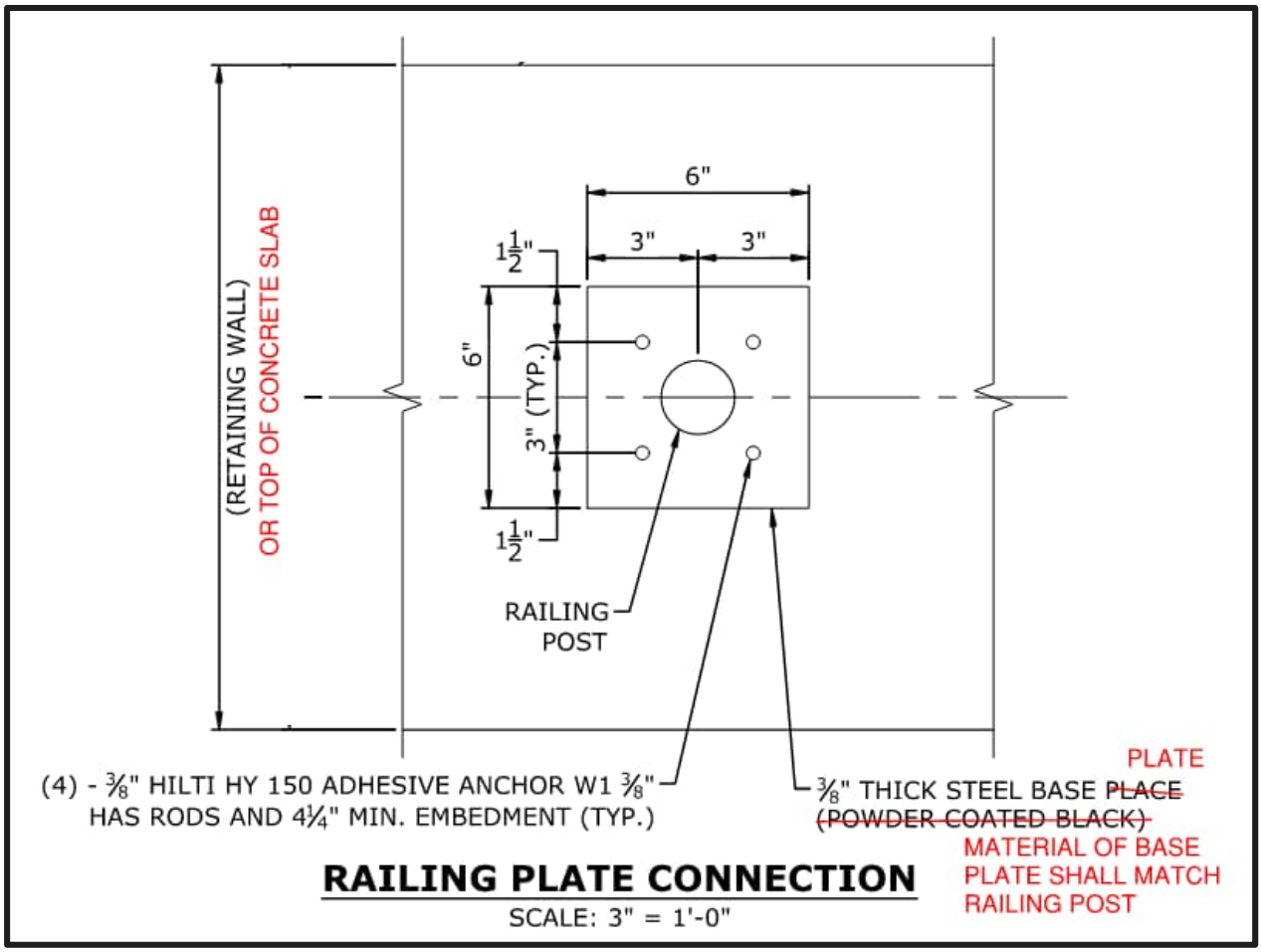
d. It appears that there is a section of railing between the proposed concrete retaining wall and the existing millwright office. Please confirm that this is the intent, provide the dimension of the railing and foundation details.

R11. a. The safety railing shall consist of a 1-1/2" Dia. Galvanized Rail that is 42" high with 1" Dia. Balusters spaced 4-1/2" o.c. The posts shall be spaced a maximum of 6 feet o.c. See the sketch provided below for railing plate connection detail.

b. It is anticipated that the manufacturer of the safety railing will have lockable gate options. For pricing purposes, please carry a $1,000 allowance to cover the cost of the gate in the Lump Sum bid price. The Contractor shall be required to provide shop drawings for review and approval.

c. Yes, the safety railing shall be galvanized steel.

d. Yes, the safety rail between the end of the retaining wall and existing Millwrights Office building is required. The on-grade section of safety railing shall be founded using 12" concrete Sonutubes that are a minimum 4 feet deep, and the railing post shall be embedded a minimum 12" into the concrete. The plans that show the safety railings are drawn to scale.



Q12. Please provide a list of the items or submittals that will be required for review and approval for FERC, to include the review timeline.

R12. A cofferdam plan shall be submitted by the contractor for review and approval. The Cofferdam Plan will require a maximum 60-day review period, which includes review by the Project Engineer, then submittal to FERC for final review and approval. FERC requires a 30-day review period that is included in the 60-day max review period. There are additional documents that Stowe Electric Department are responsible for submitting to FERC for review and approval, including but not limited to a Temporary Construction Emergency Action Plan and a Quality Control Inspection Plan. The contractor awarded the bid will need to provide minimal information that will be inserted into those documents prior to submitting them to FERC for review and approval. Construction will not be allowed to begin until FERC approval of these documents is received.

Q13. Regarding the historic scope of work, will there be any required reviews or inspections from the Vermont State Historic Preservation Office?

R13. The contractor shall submit shop drawings and/or samples of products to be used for the work items associated with the historic mill to be reviewed and approved by the Owner.

Q14. Is there a preservation consultant working on this project?

R14. Yes, the architect, Christian C. Carey Architect, P.C., shall serve as the historic preservation consultant during construction.

Q15. What species of wood is to be used for new siding on the historic mill building?

R15. The species of wood to be used for siding on the historic mill building shall be 8-inch tongue and groove vertical cedar plank siding.

Q16. Can the approximate weights for the major owner supplied components please be provided? Or at least the heaviest components for each item.

R16. Refer to the response to Question 2 and Question 4 in Section A above for weights of components supplied by Canadian Hydro Components and Obermeyer.

Q17. Page 15 of the RFB paragraph d states that only US Produced steel and iron may be used on the project. The plans specify a gate as manufactured by Fontaine Aquanox which is a Canadian company. Will Canadian manufactured metals be utilized on this project? Or are there specific items that are exempt from the Buy America provisions?

R17. Refer to the response to Question 5 in Section A for more information related to the slide gates.

Q18. Page 14 paragraph A of the RFB states that cofferdam design must be submitted to SLR and FERC 60 days prior to the installation of the cofferdam. Have conversations been had with FERC regarding this review time? How will project delays be handled if the FERC review takes longer than 60 days?

R18. Refer to the response to Question 12 in Section B for more information related to the Cofferdam Plan and related FERC requirements.

Q19. Concrete note 1 on sheet N-01 states that all concrete for the proposed sluiceway, intake structure and dam shall be "Concrete, High Performance – Class B". Is this a reference to a VTrans mix design? The concrete specification says to reference the drawings for mix design. The only other note regarding concrete mix design is on STR-06 which says "6000 PSI concrete grade beam". A VTrans Class B mix is 3500 PSI. There is no commercially available 6000 PSI ready mix concrete product in Vermont. Please clarify the concrete mix design intended to be used on this project and note that 6000 PSI ready mix concrete is not an option.

R19. All concrete used in sluiceway will be "Concrete, High Performance – Class B" with a strength of 3,500 psi. Concrete mix design used for the grade beam will be submitted by contractor and shall achieve 6,000 psi strength with a maximum aggregate size of ¾".

Q20. Please confirm that all reinforcing steel in cast in place concrete shall be epoxy coated.

R20. All reinforcing steel for cast-in-place concrete shall be epoxy coated.

Q21. What is the load rating for the top slab and grating on the new and modified existing sluiceway? Will equipment be allowed to be driven over this? Manlifts, forklifts, or similar etc.

R21. The top slab and grating are designed for a pedestrian live load of 100 psf.

Q22. STR-01 depicts W8x35 beams under the top at the new power house building.

a. What is the proposed finish for these beams, galvanized, plain black, etc?

b. How are these beams attached to the walls? Are there supposed to be anchors of some kind? The generator frame has anchor bolts and neoprene pads.

c. Are there any shear studs required on top of these beams in order to positively connect the beams to the top slab?

R22. a. All steel shall be galvanized, unless otherwise noted as stainless steel.

b. See the response to Question #9 in Section B for more information.

c. Shear studs shall not be required. The Contractor shall form and pour the top slab as shown.

Q23. Regarding the steel grating on the top slab of the sluiceway:

a. What is the proposed finish for the steel grating?

b. What is the proposed size, thickness and spacing for the grates?

c. Are these grates intended to be removable (for maintenance purposes) or are these secured in place permanently?

R23. a. See the response to Question #7 in Section B for more information.

b. See the response to Question #7 in Section B for more information.

c. Yes, the grates will be removable using clips by manufacturer.

Q24. More detail and dimensions are needed surrounding the concrete encasement around the draft tube. The only real depiction is on sheet PR-01. Section 5 on sheet STR-01 only details the top slab area. STR-02 and STR-03 do not depict enough detail on the draft tube encasement. The CHC shop drawings appear to depict a very specific size and shape slab under the draft tube, but there are no elevations or design with it. Elevations, lengths, widths, reinforcement details, and min concrete thickness would be required at a minimum in order to properly price this scope.

R24. For pricing, assume #4 bars @ 12” o.c each way with 3” clear cover. Adjust as needed to avoid anchor studs from draft tube by CHC.

Q25. Drawing STR-04, detail 1 shows the retaining wall elevation, The wall height states “varies, see elevation”. Please clarify where the elevation is depicted. If no elevation available, please provide the top of wall elevations (low and high).

R25. The top of wall elevation is 641.6 and will not vary.

Q26. Drawing PR-01 has a note to “Provide opening with grate and steel framing to access sluiceway intake”. This opening is again visually depicted on sheet STR-04, but there is no call out, dimensions or any details regarding this item. Please provide details for this opening, including but not limited to size, depth, framing, material types, finishes, encased items, attachments, etc.

R26. See the response to Question #8 in Section B for more information.

Q27. Drawing PR-02 has a note that states “Relocated Slide Gate, See Sheet PR-01”. What does the term “relocated” mean? It appears as the slide gate at this location is brand new. Is this word “relocated” just a typo.

R27. Please disregard the word “Relocated” as shown in the callout for the intake slide gate on Sheet PR-01. Yes the intake slide gate shall be new, however it will not be placed in the same location as the existing slide gate.

Q28. Drawing STR-08 note one under Structure Repair notes says that concrete repair to be paid by the square foot. The bid form appears to only have mill foundation repairs by the lump sum and there are no unit price items for foundation repairs. These items really need to be unit price items, preferably 1 item for each of the repair details. There is no real way for contractors to determine how many square feet of repairs are needed.

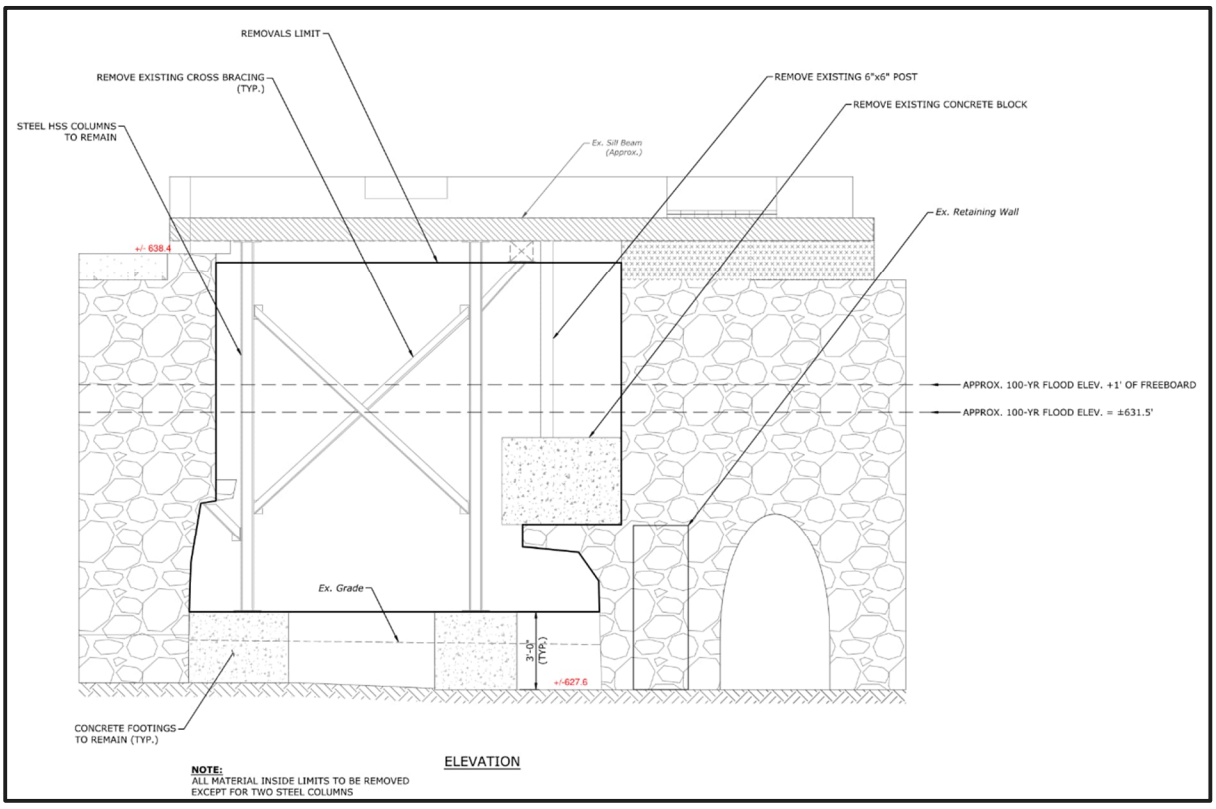
R28. See the response to Question #7 in Section A for more information.

Q29. STR-09 shows removing several cross braces and other items that appear to be structural in nature. Has any structural analysis been completed on this structure to determine whether removal of these items will effect the integrity of the building during construction?

R29. Methods of construction to not damage the existing building shall be determined by the contractor. Contractor shall remove the center X bracing just prior to construction of new concrete wall. The proposed wall is structurally adequate to brace the building.

Q30. There are no elevations on sheet STR-10. The drawing just states, "See grading and Utilities plan". Please provide some approximate elevations on the mill building to provide the relative heights as compared to the water elevations and the elevations of the new sluiceway.

R30. See sketch below and sheet STR-01 for spot elevations.



Q31. Drawing STR-011 says in several locations to "Infill void". Please provide a detail for this repair. Are we repairing this wall with concrete? Stone masonry? If a stone masonry is expected, what types of stone and mortar is expected?

R31. Refer to the responses provided in Addendum 1 and to Question 6 in Section A above for more information related to the repair of the mills foundation.

Q32. Drawings STR-011 and 012 have a repair section to "repair/infill voids". This looks like repointing the existing stone masonry wall. Is that the intent? Please provide a detail/clarification for this repair.

R32. Refer to the responses provided in Addendum 1 and to Question 6 in Section A above for more information related to the repair of the mills foundation.

Q33. If we are expected to repair the stone masonry foundations, do we need to use historic mortar matching the existing mortar? Has any testing/analysis been conducted on the mortar in order to match the existing? Is this effort required?

R33. Refer to the responses provided in Addendum 1 and to Question 6 in Section A above for more information related to the repair of the mills foundation.

Q34. Drawing STR-011 states to remove concrete wall/veneer and to repair exposed stone masonry "as needed". How are contractors expected to be able to price repairs for stone masonry that we cannot see? This scope should be dealt with as a change order in the field after the concrete has been removed or via unit price repair items with clear scope and measurement/payment provisions.

R34. Refer to the responses provided in Addendum 1 and to Question 6 in Section A above for more information related to the repair of the mills foundation.

Q35. There is a spec 03 36 00 Integral Colored Concrete. There appears to be only one mention of this colored concrete on Sheet A2.0 for the new south wall on the mill building and the new sluiceway concrete. Are there any concrete items that are not intended to receive this color? Like the dam or intake for example? Please clarify what concrete is expected to have integral color.

R35. Refer to the responses provided in Addendum 1 and to Question 6 in Section A above for more information related to colored concrete requirements.

Q36. The Rock Anchor Foundation specification section 31 68 13 has several lines that discuss the design of the rock anchors. 1.1.A states: "this work shall consist of designing, furnishing, installing and testing permanent rock anchors…" Paragraph 1.1B requires the contractor to submit their "design" for review and approval. Sheet STR-06 of the plans includes a prescriptive anchor design. Please clarify who is responsible for the anchor design.

R36. The Contractor "design" refers to assuring that their proposed installation system (i.e. drilling equipment, anchor bar supplier, grout mix, etc.) will be sufficient to install the anchors with sufficient capacity to meet the anchor design shown on the drawings.

Q37. The anchor details do not include anticipated unbonded lengths based on the bedrock elevation. It will not be possible to accurately bid the anchors without knowing the distance from the top of the dam to the anticipated foundation rock. We understand that a minimum of 10 ft of unbonded length is required. Please clarify the anticipated unbonded length at each anchor location.

R37. See the Rock Anchor Summary drawing provided as Attachment G for the information requested.

Q38. Sheet PR-02 states "performance tests, proof tests, lift off readings and lift off tests shall be performed on all installed anchors." Performing both proof testing and performance testing on all anchors will result in two complete tests on each of the anchors and is not standard practice. Generally, PTI (Post Tension Institute) recommends 2% of anchors to be performance tested and all remaining anchors to be proof tested. Please clarify that the intent is to perform either a proof test OR a performance test on all anchors and please clarify how many anchors will get a performance test.

R38. Anchors 1 through 3, as shown on the Rock Anchor Summary (see Attachment G), are 55-kip anchors, while Anchors 4 through 22 are 105-kip anchors. A performance test will be required for each type of anchor (two total). Proof tests will be required on all remaining anchors.

Q39: Rock Anchor specification 31 68 13, section 1.5G on page 6 discusses lift off tests being performed on all anchors and shall occur between 30-45 days after the anchors have been locked off. Given the extremely short in stream working window, we cannot afford to wait 30 days for these lift off tests to occur. Without knowing the specific reason for requiring the 30 day minimum, can this be reduced to 7 days minimum? Otherwise we fear there is not nearly enough time to complete the in stream dam work.

R39: Lift off tests shall be performed a minimum of 7-days after anchors have been locked off. The revised language for #2 in Section 1.5 - G in the Rock Anchor Specifications is provided below. A revised Section 31 68 13 of the Technical Specifications will be provided to the contractor awarded the bid.

> 2. Lift-off tests shall be performed at least 7 days, but no more than 30 days after the rock anchor has been set to lock-off load. The results of all lift-off tests shall be recorded.

Q40: Question 14 in addendum 1 was regarding the integral colored concrete and where it is or is not required. I do not feel that the answer is specific enough to sufficiently answer the question. The only area not specifically mentioned was the dam repair work. So just to be 100% clear, integral colored concrete is required everywhere, except for the dam work. Is that correct? The "sluiceway" is a very large section of concrete. If there are specific sections that are not required to be colored, please clearly identify them.

R40: Refer to the responses provided in Addendum 1 and to Question 6 in Section A above for more information related to colored concrete requirements.

Q41: Question 14 in addendum 1 references "coloring", "staining" and "tinting" and then trying to match the color of the existing concrete. Just so we are clear, all references to any sort of coloring of concrete (staining, tinting, etc) is required to follow specification 03 36 00 Integral Colored Concrete, which includes a very specific product, color, and requirements for a mock up. Is that correct?

R41: Refer to the responses to Question 6 in Section A above for more information related to colored concrete requirements.

Q42: As it relates to the Obermeyer Spillway Gate System, please note the following:

a. The routing of the compressed airline is shown in plan on SP-02, and referenced in utility notes 2, 3 and 4 on the same drawing. The material type and size are not identified. The compressed airline is shown in sections 1 and 2 on STR-01 and is noted as stainless steel, however the size is not identified. Obermeyer Drawing 24-2822-101, calls for '1 ea. – 1.5" schedule 10 SS fully welded (not threaded) embedded air line with a ½" condensate purge line installed'. Additionally, it is shown as sloping at a minimum of 0.1% from the bladder connection point toward the abutment. Obermeyer drawing 24-2822-125, notes that the size of the piping should be 2" or 2.5" schedule 10. Specification 35 22 13, Spillway Crest Gates, Part 2.2A, notes ' contractor supplied and installed 1-1/2" schedule 80 SS compressed air supply pipe and fittings. Please provide the stainless-steel pipe sizing, grade and schedule for the airline from the powerhouse to the dam, and each bladder connection location.

b. Drawing SP-02 note two locations for Purge Valve Lock Boxes. Obermeyer drawing 24-2822-101, includes the following notes:

1) PURGE VALVES ARE ABSOLUTELY REQUIRED FOR AFFECTIVE OPERATION OF OBERMEYER BLADDERS. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING AN APPROVED PURGE SYSTEM IS INSTALLED.
2) OWNER APPROVAL OF PURGE VALVE LOCATION IS REQUIRED AND IS OFTEN DETERMINED BY SITE OPERATIONS.

Please confirm that the count and location of the purge lockboxes is the location per the 'approved purge system'. Additionally, please confirm that the compressed air line shall be pitched to these locations, which the addition of a ½" low point tee for the addition of a ½" stainless steel schedule 10 purge line.

c. Please provide a specification for the purge valve for the purge system.

d. Please provide details for the construction of the lockbox.

e. The layout for the PLC, ACP, dual compressors and the air tank is shown on STR-03. Per specification 35 22 13:

Additionally, Contractor responsible for supplying the following:

- Interconnecting conduit, wiring, or piping.
- Compressed air line piping and associated fittings, etc.
- Purge valves and associated piping, fittings, steel security cover, etc.
- Stilling well for water level transmitter.
- Any required anchor bolt epoxy.

Please provide a one-line drawing and piping schematic for the interconnect work associated with this equipment.

R42: a. All compressed airlines shall be 2" stainless steel, schedule 10. The condensate purge lines shall be ½" stainless steel, schedule 40. All compressed air lines need to slope towards the purge valves at a minimum 0.1% slope.

b. The compressed airline routing on SP-02 shows two low points, with purge lock boxes at each location. The exact locations of the purge lockboxes are flexible, but must be at any low point in the compressed air line. All airlines must be pitched to a purge lockbox. The contractor may choose to add an additional purge lockbox if an additional low point is necessary.

c. The purge valve shall be a ¼ turn ½" ball valve.

d. Additional information will be provided in a subsequent addendum issued on or before January 30, 2026.

e. Additional information will be provided in a subsequent addendum issued on or before January 30, 2026.

Q43: Please note drawings STR-03, A1.0, A2.0, A2.1, M1 and M2 and advise on the following:

- **a. Please confirm that the ceiling height is 8'-8 -1/4" above the concrete slab as shown in section 2/A2.1.**
- **b. Please confirm that ½" GWB is the desired finish for the interior walls and ceiling of the new powerhouse.**
- **c. Please provide mounting heights for LV -1, LV -2, EF-1, MD-1, FC-1 and HP-1, as shown on M1.**
- **d. Please provide interior and exterior trim details for LV-1 and LV -2.**
- **e. Please confirm that the mounting location for HP-1, as shown on M1, is accurate.**
- **f. Please provide the reveal for the composite clapboard siding.**
- **g. Detail 2/A2,1 notes 2x10 rafters and a 2x? rafter tie. Please advise on the following:**
  - **i. Is the rafter spacing intended to match that of the wall (16" O. C.)?**
  - **ii. Please provide the size of the rafter tie.**
  - **iii. Is a connection plate required for the rafter to the wall?**
  - **iv. Would manufactured wood trusses be an acceptable alternative to rafters?**
- **h. The southeast elevation on A2.1 calls for a baffled ridge vent. We could not find a specification for this item. Please provide a specification, or a manufacturer and product number.**
- **i. Specification section 077100 Roof Specialties is provided in the bid documents, and discusses copings, gutters and downspouts, regrets and counterflashing. We could not find a location in the drawings where this specification applies. Please review and advise.**
- **j. Please confirm that the location of the Hydro Generator Service Disconnect Shown on E2 is accurate. Additionally, please provide mounting heights and requirements, as well as trim details.**
- **k. Per drawing note 3 on E2, add provide the mounting location, height, requirements as well as trim details.**

R43: a. Yes, the ceiling height is shown correctly.

b. Yes, ½" GWB is the desired finish for the interior walls and ceiling of the powerhouse.

c. Additional information will be provided in a subsequent addendum issued on or before January 30, 2026.

d. Additional information will be provided in a subsequent addendum issued on or before January 30, 2026.

e. Additional information will be provided in a subsequent addendum issued on or before January 30, 2026.

f. The composite clapboard siding shall include a 4-inch reveal. See architectural drawings for more information.

g. The rafters shall be spaced 16” o.c. to match the wall stud spacing. Rafter ties shall be 2x8. A connection plate is not required, unless required by Vermont Building Code or local ordinances. Manufactured wood trusses would be an acceptable alternative to rafters, contractor shall provide shop drawings for review and approval if proposed.

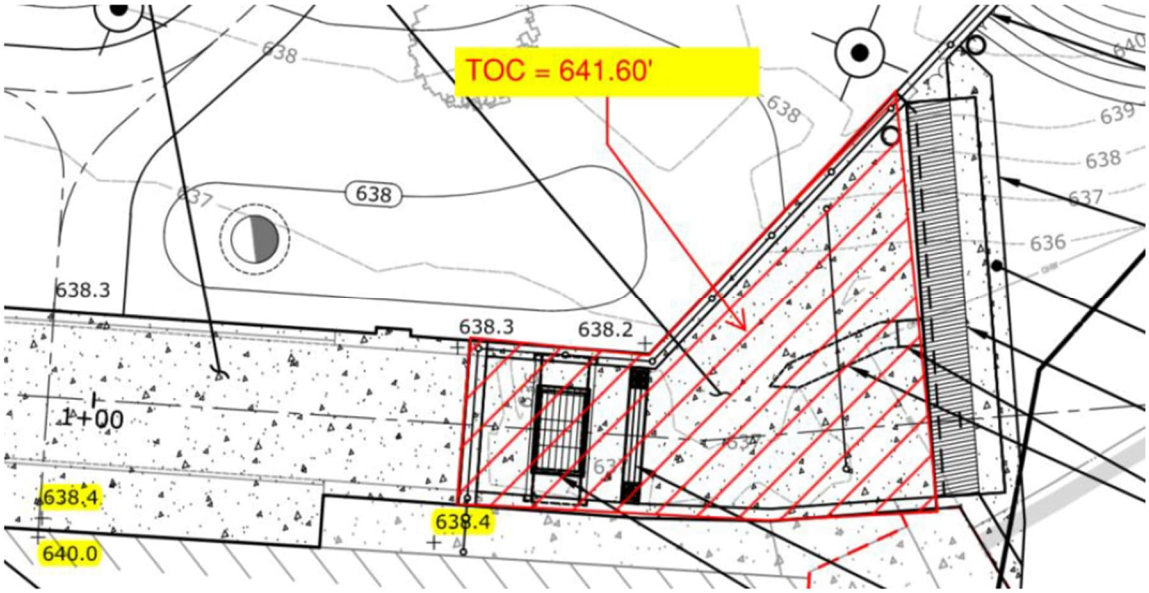
h. Information related to the ridge vent has been provided in Attachment H.

i. The Contractor shall provide flashing and counter flashing at window and door openings. The intent for section 077100 Roof specialties was to make provisions for metal prefinished drip edges (see detail A2.1) along the eave and gable ends. No reglets, gutters or downspouts are required for the Powerhouse structure.

j. Additional information will be provided in a subsequent addendum issued on or before January 30, 2026.

k. Additional information will be provided in a subsequent addendum issued on or before January 30, 2026.

Q44: Please note PR-01, SP-02 and STR-04. The elevation for the top of the intake deck is at 641.60, and the elevation at the top of the sluiceway deck is 638.40 +/-, or a difference of 3.20’. What are the details for the gate and access to the intake deck at the following location (from PR-01):



R44: A second gate to access the top deck of the intake structure will not be required. Access to the intake will be from the gate located adjacent to the Mill.

Q45: Please note drawing STR-08, note 2 under ‘Structure Repair Notes’, and Addendum 1 question & response #39, which notes that ‘additional information in response to this question will be provided in a subsequent addendum’. When looking at this subsequent information, it would be prudent to include the limits of repair on the vertical and horizontal faces of concrete faces, i.e. does the

resurfacing stop at the normal pond elevation, or other? Additionally, does it extend to the downstream surfaces of the existing mill building intake area?

R45: This work applies on all existing concrete to remain. Structure repair shall extend down to finished grade.

Q46: Please note Addendum 1, question & response #14, which discusses the locations where colored concrete is required. Per this response, please confirm the following:

a. As it relates to the new sluiceway concrete as recapped in the second paragraph of the response, please confirm that this includes the new sluiceway walls and the sluiceway deck slab.

b. Additionally, what area(s) of the powerhouse foundation and tailrace concrete walls are to have colored concrete.

c. Please confirm that Sika Color 120 G color Chart – 'Winter Beige' (C16), as noted in specification 03 36 00 Integral Colored Concrete, is the color required in all areas to received colored concrete.

R46: Refer to the response to Question 6 in Section A above for more information related to colored concrete requirements.

Q47: The top of concrete for the new intake deck steps from 641.60 to 638.40 near STA 0+78 +/-, as shown in the profile view on PR-01. Please provide details for this step, including the reinforcing needed at the intersection between the upper and lower decks and wall between them.

R47: Additional information is provided in SK-1, see Attachment F

Q48: Please provide the size of the steel plates shown in section 2 on STR-04.

R48: Exact dimensions will depend on conditions found in the field. For pricing, approximate dimensions are given for openings and a ¾" thick steel plate shall be used. Steel plate should extend 12 inches beyond openings.

Q49: Drawing E2 – the one line shows a new panel in the Mill Building and removal of an old panel and service. Nowhere does it show any branch circuits being moved from one to the other. And the panel schedule shows only one breaker for the SPD. We would like to verify there is no work in this building beyond removing the old panel and service and installing the new?

R49: Additional information will be provided in a subsequent addendum issued on or before January 30, 2026.

Q50: Drawing E2 – one line shows removal of the existing service and panel, but only shows conduit being run for power and telecom but no wire or service panel. We would like verification that there is no work there other than running the underground conduits to the building and removing the existing service.

R50: Additional information will be provided in a subsequent addendum issued on or before January 30, 2026.

Q51: On Drawing E1 under Utility Notes number 3.2 utility responsibilities number 3.2.2. It says supply a precast vault to utility spec. This is listed under utility responsibilities but is written in a way that it appears to be the electrical contractor's responsibility. Please clarify who supplies the vault.

R51: Additional information will be provided in a subsequent addendum issued on or before January 30, 2026.

Q52: Drawing E1 shows three site lights labeled with a note 1 which states chosen by architect. How should we carry this in our bid?

R52: The product information for the site lights is provided in Attachment H.

Q53: The architectural drawings show a light on the end of the powerhouse which does not show up on the electrical drawing. What is this light and who is providing it?

R53: The product information for the exterior powerhouse light is provided in Attachment H**.**

Q54: Drawing E1 shows an underground conduit for security cameras but is unclear where it needs to be run to?

a. Where do we need to have this conduit?
b. Is there any other scope involving the cameras or is it just the conduit?

R54: Additional information will be provided in a subsequent addendum issued on or before January 30, 2026.

Q55: Do you have a target date on when addendum #2 will be issued?

R55: Addendum #2 was issued on January 23, 2026. Additional responses to bidders' questions that are not answered in Addendum #2 will be provided in a third addendum issued no later than January 30, 2026.

Q56: Specification Section 101400 Signage Provides the following list of signs:

A. This section includes the following:

1. Do Not Enter signs.
2. Danger Dam Ahead signs.
3. Keep Out signs.
4. Portage signs.
5. All sign posts and mounting hardware.
6. All other signs shown on plans.

The only sign locations that we have found are shown on drawing L1.2 and L1.3 identifies 3 trail sign posts (E/L3.0), one info sign (F/L3.0) and a sign mounted on the bridge abutment (G/L3.0). Details E & F on L3.0, note that the 'sign by owner, NIC'. Please confirm that the portage sign, G/L3.0 is the only sign that the contractor needs to procure and mount.

R56: Yes, the portage sign shall be supplied and mounted by the contractor. The Trail Sign shall be supplied and installed by the Owner, however the contractor shall supply and install the sign post in the locations shown on the plans.

Q57: Please note SED General Conditions and Contract Requirements, regarding indemnification and USDA RUS Construct Contract Article IV.1d ; Would the

Owner be willing to remove the Duty to Defend from the indemnification provision?

R57: SED cannot unilaterally remove or amend a provision in the RUS form contract without approval from USDA RUS. SED can initiate a conversation with USDA RUS after the preferred bidder is offered the project, but before the parties sign the contract.

Q58: Please note USDA RUS Construct Contract Article V.2 – Liquidated Damages; This section notes that liquidated damages apply to the agreement, however the amount of the damages is not listed. Please review and advise.

R58: SED has provided milestones for this Project and bidders are encouraged to submit clear timelines showing how they intend to meet those milestones. SED will negotiate the liquidated damages clause based on the milestones and in good faith with the contractor offered the Project but before the Contract is signed.

Q59: Please see Construction Sequence Note, Stage 1A 1;

STAGE 1A:

1. TEMPORARILY BLOCK THE OPENING TO THE MILL INTAKE STRUCTURE LOCATED AT THE NORTH END OF THE DAM TO ALLOW WORK TO TAKE PLACE ON THE EXISTING LOW-LEVEL OUTLET. ALL INFLOW SHALL CONTINUE TO PASS OVER THE EXISTING CONCRETE SPILLWAY.

Please provide the dimensions of the opening that we need to temporarily block.

R59: The opening on the upstream face of the concrete mill intake structure is approximately 12 feet wide by 8 feet high.

Q60: Please see Construction Sequence Note, Stage 1A 4;

4. ONCE SEDIMENT REMOVAL IS COMPLETE, OPEN THE EXISTING LOW-LEVEL OUTLET SLIDE GATE TO BE USED FOR WATER CONTROL DURING STAGE I OF CONSTRUCTION. WHEN FULLY OPEN, THE CAPACITY OF THE LOW-LEVEL OUTLET IS AS FOLLOWS:

Please confirm that this low-level gate is fully functional without additional means of operation provided by the contractor.

R60: The existing low-level gate is not fully functional. Additional means to operate the existing gate should be anticipated. Alternative means for initial drawdown of the impoundment as proposed by the Contractor will be considered.

Q61: Please see Construction Sequence Note, Stage 1B 5;

STAGE 1B:

5. REMOVE THE TEMPORARY BLOCKAGE FROM IN FRONT OF THE MILL INTAKE STRUCTURE IN CONTROLLED INCREMENTS TO BEGIN DRAWDOWN OF THE IMPOUNDMENT UNTIL A MINIMUM WATER SURFACE ELEVATION OF 629.0 FEET NAVD88 IS ACHIEVED AND THE EXISTING SPILLWAY CREST IS ABOVE WATER. DRAWDOWN SHALL BE PERFORMED SLOWLY TO MINIMIZE THE POTENTIAL FOR SEDIMENT MOBILIZATION. A MAXIMUM RATE OF 2.0 FEET PER DAY SHALL BE USED TO DRAW DOWN THE IMPOUNDMENT. EXPOSURE OF THE SEDIMENT THAT HAS ACCUMULATED UPSTREAM OF THE DAM ALONG THE SOUTHERN CHANNEL BANK IS ANTICIPATED DURING INITIAL DRAWDOWN.

Is the contractor limited to a maximum elevation of drawdown/hr. or can the 2-foot change occur at one time in the one-day period.

R61: The maximum allowable drawdown rate shall be 1.0 foot per day or ½" per hour. Rapidly releasing the daily maximum of 1-foot over a short period is prohibited.

Q62: Please see Construction Sequence Note, Stage 1B 6.

> 6. ONCE DRAWDOWN IS COMPLETED, REMOVE THE TOP OF EXISTING CONCRETE SPILLWAY TO ELEVATION ±630.3 FEET NAVD88 AS PROPOSED FOR DAM REHABILITATION, TO THE EXTENT PRACTICABLE, PRIOR TO INSTALLING THE SOUTHERN COFFERDAM. LOWER A MINIMUM LENGTH OF 75 FEET OF THE CREST AT THE NORTHERN END OF THE EXISTING SPILLWAY. THE LOWERED SPILLWAY SECTION WILL BE USED, IN CONJUNCTION WITH THE OPENED LOW-LEVEL OUTLET, AS WATER CONTROL DURING THE NEXT STAGE OF DAM REHABILITATION.

Please confirm that the intent of this sequential note, is to remove the 75 feet of dam crest without having any sort of cofferdam in place.

R62: It is anticipated that the impoundment will have been drawn down prior to removal of the dam crest, therefore the crest will be above water for the removal of the existing concrete. Alternative sequencing and means for water control will be considered as proposed by the Contractor. The Contractor awarded the bid shall be required to submit a cofferdam plan and water control plan for review and approval.

C. REVISIONS TO CONTRACT DOCUMENTS

The following modifications to the bid documents shall be made part of the official bid package and supersede any previous documentation. Revised sketches have been provided or attached herein:

1. A modification to the fall protection rail located on the sluiceway intake structure has been made. This applies to the detail shown on STR-05 and plan view on sheet STR-04. The overall length of the rail has been extended from ±16'-0" to ±19'-0", and shall have three posts instead of two. Two of the posts shall be installed in the slab above the outer walls of the sluiceway intake, and the third installed above the central pier. See plan view sketch provided in the response to Question #10 in Section B for the proposed modification. The railing plate connection detail on Sheet STR-05 has also been updated, see the response to Question #11 in Section B for the proposed modification.
2. After additional discussion with the Historic Preservation consultant that was involved in the Section 106 - National Historic Preservation Act of 1966 review, the product that shall be used where colored concrete is required is the Sika French Grey (C14) product as shown on the SikaColor-120 G Granular Integral Colors color chart or approved equal. A mock up using this product shall be required to be reviewed and approved by the Owner. Additional information related to the colored concrete requirements are provided in response to questions in Section A and Section B.

D. ATTACHMENTS

A – Revised Sheet EX-02

B – CHC Preliminary Transportation Plan

C – CHC Commissioning Overview Plan

D – Slide Gate Specifications

E – Revised Bid Form

F – SK-1

G – Rock Anchor Summary

H – Architectural Components